lce (wind),  $I_W = 1.00$ Live Loads:

20 psf uniform, 300 lb concentrated 2nd floor.

Ground Snow Load, (Pa): 10 psf

Wind Design Data: Ultimate Design Wind Speed, Vult (3-second gust) 132 mph (ASCE 7-10/ 2018 NC State Building Code) Internal pressure coefficient (GC<sub>pi</sub>) = ±0.18 [Enclosed Building]

Exposure Category C Wind Base Shears (for MWFRS):  $V_X = 28 \text{ kips}$ [At addition only]

\*Note: All exterior component and cladding materials will be designed and anchored to resist 132 m.p.h component and cladding wind forces determined according to the NC State Building Code for the Exposure Category indicated above. Provide all bracing and connections necessary to transfer lateral loads to structure.

Earthquake Design Data:

Seismic Design Category C Mapped MCE<sub>R</sub> Spectral Response Acceleration Parameters, S<sub>S</sub> = 23.0 %g

MCE<sub>R</sub> Spectral Response Acceleration Parameters, S<sub>MS</sub> = 36.8 %g Design Spectral Response Acceleration Parameters,  $S_{DS} = 24.5 \text{ %g}$   $S_{D1} = 15.8 \text{ %g}$ Site Classification = D

Seismic Force Resisting System:

Bearing Wall X Building Frame

Moment-Resisting Frame

Dual w/Special Moment Frames

Dual w/Intermediate Moment Frames Shear Wall-Frame Interactive System w/Ordinary Reinforced Moment Frames and Ordinary Reinforced Concrete Shear Walls

Cantilevered Column Systems Steel Systems Not Specifically Detailed for Seismic Resistance, Excluding Cantilever Column Systems

Response Modification Coefficient, R = 3

Seismic Response Coefficient, C<sub>S</sub> = 0.09 Seismic Base Shear V = 11 kips

Analysis Procedure: Simpified

Equivalent Lateral Force Modal

Lateral Design Control: Earthquake\_\_\_\_ Wind X

Note: Special Inspections are required for this project in accordance with the North Carolina State Building Code Section 1705. See Statement of Special Inspections on Sheet S0.02 for required structural inspections.

Materials A. Concrete

f'c = 4000 psi Reg. wt. U.N.O. f'c = 4500 psi Reg. wt. at pedestals (air content 6% ± 1.5%).

f'c = 4000 psi Lt. wt. (120 pcf fresh density +/- 5 pcf max) at elevated slabs on metal deck.

All concrete to have max water/cement ratio of 0.45. ASTM A-615 Grade 60

B. Reinforcing Steel C. Welded Wire Fabric ASTM A-1064 D. Structural Steel ASTM A-36 U.N.O.

ASTM A-992 at all beams and columns E. Hollow Structural Sections(HSS) Rectangular - ASTM A-500 Grade C,  $F_V = 50$  ksi

ASTM F-3125, Gr.A-325N w/ASTM A-563 heavy hex nuts and ASTM F-436 washers

G. Weld Electrodes Composite Deck: 40 ksi H. Steel Deck Roof Deck: 33 ksi min

ASTM A-653 galvanized ASTM F1554(Gr.36) w/ ASTM A-563 heavy hex Anchor Rods

J. Expansion Bolts Hilti Kwik Bolt III (or equal)

Hilti HIT-HY 270 (or equal) for anchorage to masonry K. Epoxy Anchoring System Hilti HIT-RE 500 V3 (or equal) for anchorage to concrete

Foundations

A. Allowable soil bearing 2000 psf. Site will be proofrolled, densified and/or undercut, tested, and repaired according to subsurface exploration report by Geotechnologies, Inc. dated October 30, 2015 [Geotechnologies Inc. Project #1-15-0630-EA].

before placing reinforcing in footing excavations.

The general contractor will coordinate and lower wall and column footings where required to be below any new or existing utility lines. Notify Arch./Engineer of any footing adjustment required before beginning work. In no condition will utilities be allowed under

Compact bottom of all footing excavations with a vibratory hand operated compactor

column footings. Sides of concrete pedestals are to be formed.

No heavy equipment will be used within 8 feet of existing building elevator pit walls. All concrete work will comply with ACI 301 "Specifications for Structural Concrete" and ACI 318-14 "Building Code Requirements for Structural Concrete" unless otherwise noted on the drawings and specifications.

4. Superstructure

Structural steel fabricator is to be (BU) AISC Certified. Structural steel erector is to be (CSE) AISC Certified. Submit certifications.

All welders, shop and field, will be certified for the type of work involved. Submit

The structural steel fabricator will prepare shop drawings under the supervision of a connection design engineer registered in North Carolina. Submit calculations for connection design along with shop drawings.

D. All shop drawings will be checked by the fabricator before being submitted to the Architect/Engineer for review. When submitting electronic shop drawings, one (1) hard copy must be provided to the structural engineer before review can begin.

At construction completion, the steel fabricator will submit a letter sealed by the connection design engineer registered in North Carolina stating that all connections have been designed and detailed for the shear values, tensile loads and capacities shown on the structural drawings. Any required field modification

sketches are to be included with letter. Structural steel connections will be designed by the fabricator's NC registered engineer unless specifically detailed on the structural drawings. All connections are to be designed as simple connections except where indicated otherwise on plans or in sections. Design connections per AISC Manual of Steel Construction (ASD) for shears indicated in the Framed Beam Connection Table(s) unless higher shears are indicated on framing plans or details. In addition, design connections for any tension indicated on framing plans, sections or schedules. Connect girders to HSS columns with thru-plates as determined by connection design engineer. Do not use slotted holes for axially loaded beams or bracing connections without written approval from the Structural

Engineer before preparing shop drawings. Connections: Shop - Bolted or Welded Field - Bolted U.N.O.

Bolts will be 3/4" diameter, Gr.A-325N, unless otherwise noted. Larger diameter bolts may be required by fabricator's connection design engineer at some locations such as vertical bracing connections or girder connections to HSS columns. Fabricator's connection design engineer and detailer are to clearly indicate on shop drawings all locations where larger diameter bolts are required. All bolts will be tightened by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench with a minimum of 200 foot pounds of torque and a maximum of 250 foot pounds of torque for a snug tight condition.

U.N.O. bolt threads can be either cut or rolled. If threads are rolled, do not reduce bolt diameter. Make adjustments on shop drawings for increased shank diameter at thread

All structural bolts will have heavy hex nuts. Unless shown otherwise all field welds will be 3/16 inch fillet welds.

All splices at perimeter roof edge angles or plates will be butt welded all around to provide roof diaphragm.

Provide screed angles around all floor openings. General Contractor to verify all existing dimensions, elevations, and conditions before fabricating steel. Notify Architect and Engineer of any significant discrepancies. All detailing, fabricating, and erection shall conform to the following specifications (latest

editions), except as otherwise indicated: AISC 360-10 - "Specification for Structural Steel Buildings." AISC 303-10 - "Code for Standard Practice for Steel Buildings and Bridges." Paragraph 4.4.1 of the above code is hereby modified by deletion of

the following sentences: a) "Confirmation that the Fabricator has correctly interpreted the Contract Documents in the preparation of those submittals"; b) "Confirmation that the Owner's Designated Representative for Design has reviewed and approved the Connection details shown on the Shop and Erection Drawings and submitted in

in accordance with Section 3.1.2, if applicable"; AWS D1.1 - "Structural Welding Code - Steel." SJI - "Code of Standard Practice for Steel Joists and Joist Girders."

Steel Joist Institute SDI - "Code of Standard Practice." Steel Deck Institute.

Shop Painting - One coat of fabricator's standard typical U.N.O. Omit paint on all structural steel that is to be sprayed with fireproofing or encased in concrete; omit paint on surfaces where shear studs will be field-applied. Hot-dip galvanize steel according to ASTM A-123 where galvanizing is required. Touch up welds and damaged areas with a field applied galvanizing according to ASTM A780.

General Contractor shall have sole responsibility for site safety. Fabricator and Erector shall review Contract Documents and if the structure, as shown on those documents, is in conflict with the requirements of any erection safety regulations, Fabricator shall notify Structural Engineer prior to commencing erection shop drawing production. If Fabricator and/or Erector fail to notify Structural Engineer, as stated above, they shall become responsible for all costs for correcting such conflicts with the requirements of any and all safety regulations.

General Contractor will be responsible for properly guying and bracing the structure to resist live, dead, wind, and construction loads during construction. An unstable condition exists until all the building components are in place. Columns and walls are not designed as free-standing and must be adequately braced.

The General Contractor will coordinate and verify size, location, type, and direction of all pads, depressions, bolts, sleeves, anchors, openings, etc. to be set or cast in concrete or masonry prior to placement. All penetrations of walls and floors shall be approved by the architect and structural engineer if not indicated on the structural drawings. All penetrations shall be sleeved with structural steel sleeves. Sleeves shall be supplied by the trade requiring the penetration.

General Contractor is responsible for coordinating locations of braces, bridging, and miscellaneous framing to avoid conflict with ducts, plumbing, or any other utility.

Steel Deck

All deck welders are to be certified for the type of work involved. Submit certifications. All deck will meet the requirements of the Steel Deck Institute and will be continuous over three or more spans unless noted otherwise.

Roof deck will be 1.5", 20 gauge, G60 galvanized wide rib (Type B) steel deck New Millennium or equal) and will be welded to supporting beams and angles with 5/8"diameter puddle welds (U.N.O.):

- intermediate supports: 6" o/c (36/7 Pattern) - individual deck ends (deck ends at building perimeter and deck end laps): 6"o/c - deck sides at building perimeter and ridges: 12"o/c. Provide continuous 20 gauge

filler sheet if deck is cut. Provide four (4) #10 tek side lap screws for 1.5" roof deck.

Composite steel floor deck will be 2", 20 gauge, with G60 Galv. (type 2.0CD by New Millennium or equal) with 5/8" diameter puddle welds 12"o/c at ea. supporting member. Side laps and deck sides at perimeter will be fastened at midspan or 3'-0" o/c maximum between supports, whichever distance is smaller. No button punching allowed. Provide deck support angles around columns, at dropped beams, and any other

required locations. Provide an angle frame to support roof deck around all roof deck penetrations greater than 6" wide, including sump pans at roof drains. See typical detail.

Provide 4" minimum end lap for roof decks. Butt ends of 2" composite floor deck over

6. Utilities and Architectural Components

The General Contractor will coordinate and lower new wall and column footings where required to be below any new or existing utility lines. Notify Arch./Engineer of any footing adjustment required before beginning work. In no condition will utilities be

allowed under new or existing column footings. Do not undermine new or existing footings when placing utilities. Provide soil shoring as

needed to prevent undermining new or existing footings. No electrical conduit runs will be located in the elevated floor slab unless approved in

writing by Architect and Structural Engineer.

The General Contractor will coordinate and verify size, location, type, and direction of all pads, depressions, bolts, sleeves, anchors, openings, etc. to be set or cast in concrete or masonry prior to placement. All penetrations of walls and floors shall be approved by the architect and structural engineer if not indicated on the structural drawings. All penetrations shall be sleeved with structural steel sleeves. Sleeves shall be supplied by the trade requiring the penetration.

Do not core through or otherwise damage existing building concrete beams, concrete joists, concrete columns or footings when placing new utilities. Do not core or cut through tapered edges of concrete joists.

Do not core through or otherwise damage structural framing or footings when placing

G. Do not suspend bulkheads, utilities, etc. from the underside of concrete beams and concrete joists. Attachment should be made to the side of the concrete structural member (coordinate anchorage location with concrete reinforcing locations - Do Not Damage Reinforcing).

Do not support suspended ceilings, light fixtures, ducts, metal studs, or other items from steel floor or roof deck. A 50 lb. max load per 4 ft x 4 ft square area is acceptable hanging from the composite floor slab. Anchors attached to the composite metal floor deck are not acceptable. The hangers supporting the load from the floor slab should extend well into the concrete and should be installed before the concrete is poured to ensure that the load is supported by the concrete and not by the metal deck. Slots cut into the floor deck for the placement of hangers should be as small as possible and should be cut parallel to the flutes. Slots in the deck shall not be cut perpendicular to the

All exterior cladding and building components will be designed and anchored to resist 132 m.p.h. wind forces determined according to the NC State Building Code for the Exposure Category indicated in these General Notes. Provide all bracing and connections necessary to transfer lateral loads to structure.

7. Miscellaneous

The structural drawings take precedence when a conflict occurs between the structural drawings, the architectural drawings, the PM&E drawings, and the specifications concerning a structural item. Notify Architect and Engineer in writing for clarification prior to bidding. The term unless noted otherwise (U.N.O.), means unless noted otherwise on the **structural drawings**. If there is a conflict on the structural drawings or specifications the most stringent requirement will control.

General Contractor to verify all existing building dimensions, elevations, and details with the field conditions. Notify the architect and engineer of any significant discrepancies. General Contractor is responsible to coordinate locations of braces, bridging, and

miscellaneous framing to avoid conflict with ducts, plumbing, or any other utility. General Contractor to notify our office during major phases of construction so observations can be made by our office as required.

The General Contractor will be responsible for properly guying and bracing the structure to resist live, dead, wind, and construction loads during construction. An unstable condition exists until all the building components are in place. Columns and walls are not designed as free-standing and must be adequately braced.

All shop drawings will be checked by fabricator before being submitted to the Architect/Engineer for review. When submitting electronic shop drawings, one (1) hard copy must be provided to the structural engineer before review can begin.

See architectural drawings for all required floor depressions, floor toppings, and floor slopes not shown. Maintain floor slab thickness shown on the drawings for elevated floor slabs on metal deck. Do not exceed specified floor slab thickness.

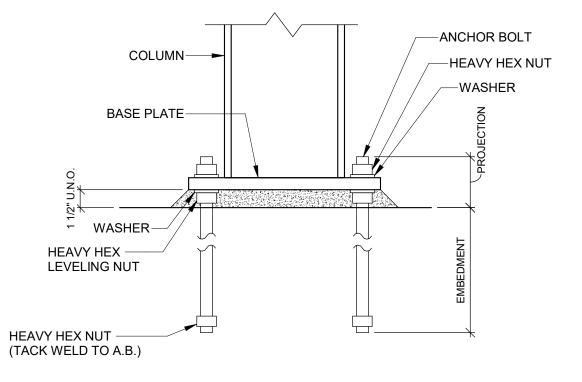
No loads will be placed on elevated floor slabs until concrete reaches 75% design All concrete work shall conform to ACI 301 and ACI 318-14, unless otherwise noted on the drawings and specifications.

K. Use type "B" laps for all reinforcing lap splices U.N.O.

8. Delegated Design

Structural steel connections will be designed by the fabricator's NC egistered engineer unless specifically detailed on the structural drawings. This includes framed beam connections and any vertical bracing

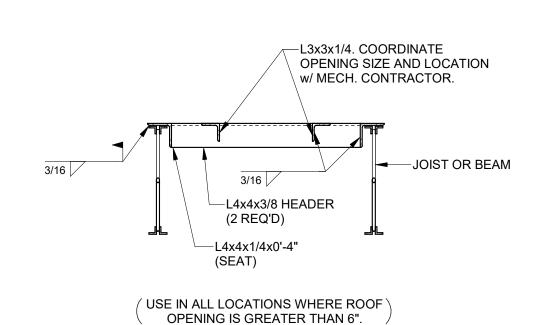
connections. Submit sealed calculations. See architectural drawings for exterior metal stud walls. Metal stud walls and their attachment to structure are to be designed by an Engineer registered in NC for 132 m.p.h. component and cladding wind forces. Metal stud design is to include headers, sills, parapets, jamb studs at openings and connections of all components to transfer loading to structure. Limit horizontal deflection of studs to L/360.



TYP. COLUMN BASE WITH LEVELING NUTS NOT TO SCALE

ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION
ALUM.	ALUMINUM	JT.	JOINT
ARCH.	ARCHITECT	LLH	LONG LEG HORIZONTAL
BLDG.	BUILDING	LLV	LONG LEG VERTICAL
BM.	BEAM	LVL	LAMINATED VENEER LUMBER
B.O.D.	BOTTOM OF DECK	MAX.	MAXIMUM
B.O.F.	BOTTOM OF FOOTING	MECH.	MECHANICAL
BOTT.	BOTTOM	MFG.	MANUFACTURER
BRG.	BEARING	MIN.	MINIMUM
B.S.	BOTH SIDES	N.S.	NEAR SIDE
CHK'D.	CHECKED	N.T.S.	NOT TO SCALE
C.J.	CONTROL JOINT	O/C	ON CENTER
CLR.	CLEAR	OPP.	OPPOSITE
C.M.U.	CONCRETE MASONRY UNITS	REG.	REGULAR
COL.	COLUMN	REINF.	REINFORCING
CONC.	CONCRETE	REQ'D.	REQUIRED
CONN.	CONNECTION	RF.	ROOF
CONSTR.	CONSTRUCTION	RM.	ROOM
CONT.	CONTINUOUS	SHT	SHEET
DWGS.	DRAWINGS	SQ.	SQUARE
EA.	EACH	S.S.	STAINLESS STEEL
E.F.	EACH FACE	STD.	STANDARD
EL.	ELEVATION	STIFF	STIFFENER
ELEV.	ELEVATION	THICK.	THICKENED
E.S.	EACH SIDE	THR'D.	THREADED
E.W.	EACH WAY	T & B	TOP AND BOTTOM
EXIST.	EXISTING	T & G	TONGUE & GROOVE
EXP.	EXPANSION	T.O.B.	TOP OF BEAM
FDN.	FOUNDATION	T.O.F.	TOP OF FOOTING
F.S.	FAR SIDE	T.O.P.	TOP OF PIER
GALV.	GALVANIZED	T.O.S.	TOP OF SLAB
G.C.	GENERAL CONTRACTOR	T.O.W.	TOP OF WALL
GEN.	GENERAL	TYP.	TYPICAL
G.T.	GIRDER TRUSS	U.N.O.	UNLESS NOTED OTHERWISE
H.C.	HOLLOW CORE	W.P.	WORK POINT
HORIZ.	HORIZONTAL	WT.	WEIGHT
HORZ.	HORIZONTAL	W.W.F.	WELDED WIRE FABRIC

SCHEDULE OF ABBREVIATIONS NOT TO SCALE



**ROOF OPENING FRAME** 

3/4" = 1'-0"

FLEMING

1004 HAY STREET

FAX 910-433-2604

ASSOCIATES, PA

FAYETTEVILLE, NC 28305

NC LICENSE NO. C-2828

FRAMED BEAM CONNECTION TABLE				
BEAM DEPTH	MINIMUM LENGTH ANGLES EA. SIDE (INCHES)	BEAM SHEAR EACH END (KIPS) *		
8	5½	15		
10	5½	15		
12	5½	20		
14	8½	24		
16	8½	30		

USE SHEAR VALUES SHOWN TO PROPORTION CONNECTIONS EXCEPT WHERE HIGHER BEAM REACTIONS ARE SHOWN ON FRAMING PLANS AND DETAILS OR CONNECTIONS ARE SPECIFICALLY DETAILED. MINIMUM LENGTH ANGLES APPLY TO DOUBLE ANGLE CONNECTIONS. PLATE SHEAR TABS ARE TO BE DESIGNED FOR BEAM SHEAR VALUES SHOWN EXCEPT WHERE DETAILED OTHERWISE ON THE DRAWINGS. NO SINGLE ANGLE CONNECTIONS ARE PERMITTED UNLESS SPECIFICALLY NOTED OTHERWISE.

FRAMED BEAM CONNECTION TABLE

NOT TO SCALE

Architecture

654 Hay Street, Suite Fayetteville, NC 2830 Phone: E-Mall: aordoniohnsonarchitecture.co

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SHEET NUMBER

GENERAL NOTES NOT TO SCALE S0.01

SHEET NAME NUMBER GENERAL NOTES & SCHEDULES S0.02 SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS S1.00 PARTIAL FOUNDATION PLAN - CONCOURSE B S2.00 PARTIAL LEVEL 2 FLOOR FRAMING PLAN - CONCOURSE B S3.00 PARTIAL ROOF FRAMING PLAN - CONCOURSE B S4.00 FOUNDATION SECTIONS & DETAILS S5.00 FLOOR SECTIONS S6.00 ROOF SECTIONS

STRUCTURAL SHEET LIST

# SCHEDULE OF STRUCTURAL SPECIAL INSPECTION SERVICES

Building Risk Category III

The following are the structural special inspection categories for this project. See the tables on this sheet for the special inspections recommended to be performed under each category. The special inspector should be familiar with the requirements of the NC Building Code Chapter 17 and the items in each category that require special inspections. See specifications for required testing not included on this sheet:

- Soils

- ☐ Open-Web Steel Joists and Joist Girders
- ☐ Cold-Formed Steel Trusses Spanning 60 ft or Greater
- Masonry Construction
- Wood Construction
- ☐ Retaining Walls Exceeding 5 ft of Unbalanced Backfill Height
- Driven Deep Foundations
- ☐ Cast-In-Place Deep Foundations
- ☐ Helical Pile Foundations
- ☐ Special Inspections for Wind Resistance
- ☐ Special Inspections for Seismic Resistance
- ☐ Structural Observations for Seismic Resistance by Registered Design Professional
- ☐ Structural Observations for Wind Requirements by Registered Design Professional

The Owner or the Owner's Agent is to hire a special inspection and testing agent to oversee, record and document all indicated special inspections according to the requirements of the NC Building Code Chapter 17. The inspection and testing agents shall be engaged by the Owner or the Owner's Agent and not by the Contractor or the Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The inspection and testing agents are to be certified for the type of work involved. Tests and inspections indicated will be based upon the requirements of the applicable building codes and the contract documents (full procedures for required testing and inspections are not included in these tables).

REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS			
		FREQUENCY OF INSPECTION	
	VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODIC DURING TASK LISTED
1.	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.		X
2.	Verify excavations are extended to proper depth and have reached proper material.		X
3.	Perform classification and testing of controlled fill materials.		Х
4.	Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	X	
5.	Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.		Х

Note: The approved geotechnical report and the construction documents shall be used to determine compliance (the most stringent controlling). During fill placement, the special inspector shall verify that the proper materials and procedures are used in accordance with the provisions of the approved geotechnical report and construction documents.

## REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

Inspections shall verify general compliance with the design documents and approved submittals as well as referenced code sections

		FREQUENCY OF INSPECTION		REFERENCED STANDARD
VERIFICATION AND INSPECTION		CONTINUOUS DURING TASK LISTED	PERIODIC DURING TASK LISTED	
1.	Inspection of reinforcing steel and placement for general compliance with the design documents and approved submittals.		Х	ACI 318: Ch.20, 25.2 25.3, 26.6.1-26.6.3
2.	Inspect anchors cast in concrete for general compliance with the design documents (including grade, diameter, gage, and embedment depth).		х	ACI 318: 17.8.2
3.	Inspect anchors/rebar post-installed in hardened concrete members. Verify installation procedure is in strict compliance with the manufacturer's instructions. Obtain installer's certification for installation of adhesive anchors/rebar before work begins.  a. Adhesive anchors/rebar installed in horizontally or upwardly inclined orientations to resist sustained	X		ACI 318: 17.8.2.4
	tension loads. b. Mechanical anchors and adhesive anchors/rebar not define in 3.a.		X	ACI 318: 17.8.2
4.	Verify use of required design mix.		x	ACI 318: Ch.19, 26.4.3, 26.4.4
5.	Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete. Verify compliance.	х		ASTM C 172 ASTM C 31 ACI 318: 26.4, 26.12
6.	Inspection of concrete placement for proper application techniques.	Х		ACI 318: 26.5
7.	Verify maintenance of specified curing temperature and techniques.		X	ACI 318: 26.5.3-26.5.5
8.	Verification of in-situ concrete strength prior to removal of shores and forms from beams and structural slabs.		Х	ACI 318: 26.11.2
9.	Inspect formwork for shape, general location and dimensions of the concrete member being formed.		х	ACI 318: 26.11.1.2(b)
10.	Inspect footing excavations for shape, general location and dimensions of the concrete member being formed.		х	

#### REQUIRED SPECIAL INSPECTIONS OF STRUCTURAL STEEL CONSTRUCTION

Inspections shall verify general compliance with the construction documents, approved submittals, and AISC 360.

		FREQUENCY OF INSPECTION		
VEF	RIFICATION AND INSPECTION	CONTINUOUS DURING TASK LISTED	PERIODIC DURING TASK LISTED	
1.	Obtain fabricator's AISC certification and submit a copy to the <i>Building Official</i> .			
2.	Material verification of structural steel components (including fasteners).		х	
3.	Verify compliance with the details shown on the construction documents, such as braces, stiffeners, and member locations.		Х	
	<u>Exception</u> : Special Inspection of railing systems composed of structural steel elements is limited to welding inspection of the welds at the base of cantilevered rail posts.			
4.	Verify proper application of joint details at each connection.	X Inspect all	-	
5.	<ul> <li>Structural steel welding:</li> <li>a. Inspection Tasks Prior to Welding (QA tasks listed in AISC 360 Table N5.4-1).</li> <li>b. Inspection Tasks During Welding (QA tasks listed in AISC 360 Table N5.4-2).</li> <li>c. Inspection Tasks After Welding (QA tasks listed in AISC 360 Table N5.4-3).</li> </ul>	As indicated in the referenced tables of AISC 360		
	<ul> <li>d. Nondestructive testing (NDT) of welded joints:</li> <li>1) Complete penetration groove welds 5/16" or greater in Risk Category III or IV.</li> </ul>	UT on 100%, r 25% per AISC		
	Complete penetration groove welds 5/16" or greater in Risk Category II.	UT on 10% ma 100% per AIS0		
	<ol> <li>Fabricator's NDT reports when fabricator performs NDT.</li> </ol>			
6.	Structural steel bolting:  a. Inspection Tasks Prior to Bolting (QA tasks listed in AISC 360, Table N5.6-1).  b. Inspection Tasks During Bolting (QA tasks listed in AISC 360, Table N5.6-2).  c. Inspection Tasks After Bolting (QA tasks listed in AISC 360, Table N5.6-3).	As indicated in the referenced tables of AISC 360		
7.	Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	As indicated in the referenced table of AISC 360		
8.	At completion of fabrication, obtain a certificate of compliance from performed in accordance with the approved construction documents.			
9.	<ol> <li>At completion of erection, obtain a certificate of compliance from the erector stating that the materials supplied and work performed by the erector are in accordance with the approved construction documents and submit it to the <i>Building Official</i>.</li> </ol>			

### REQUIRED SPECIAL INSPECTIONS OF **COLD-FORMED STEEL DECK**

Inspections shall verify general compliance with the construction documents and approved submittals. Special Inspections and qualification of welding inspectors for cold-formed steel floor and roof deck shall be in accordance with the quality assurance inspection requirements of the "QA/QC Standard for Quality Control and Quality Assurance for Installation of Steel Deck" by the American National Standards Institute / Steel Deck Institute.

#### **VERIFICATION AND INSPECTION**

- 1. Obtain fabricator's SDI certification and submit a copy to the *Building Official*.
- 2. Verify deck materials are represented by appropriate mill certifications.

design documents and applicable referenced standards.

4. Installation of mechanical fasteners in accordance with SDI C, SDI NC, SDI RD, and manufacturer's instructions.

3. Field welding of deck in accordance with AWS D1.3, SDI C, SDI NC, and SDI RD.

- 5. Steel deck installation in accordance with the construction documents, installation drawings, shop drawings,
- Scope of inspections shall comply with the quality assurance inspection requirements of Appendix 1 of the "QA/QC Standard for Quality Control and Quality Assurance for Installation of Steel Deck" by the American National Standards Institute / Steel Deck Institute.
- 8. At completion of fabrication, obtain a certificate of compliance from the fabricator stating that the work was performed in accordance with the approved construction documents and submit it to the Building Official.

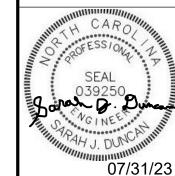




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ayetteville Author REVIEWED BY: PROJECT NO: <u>23-58</u> NOTES: \_\_\_\_

REVISIONS SHEET NUMBER

1. REFERENCED EL.0'-0" = EXISTING BUILDING LEVEL 2 FINISH FLOOR [194.19]. TOP OF FOOTING ELEVATIONS SHOWN ON FOUNDATION PLAN ARE REFERENCED TO LEVEL 1 FINISH FLOOR AT CONCOURSE B [182.19]. 2. SEE GENERAL NOTES AND TYPICAL DETAILS ON SHEET S0.01. 3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN. 4. SEE ARCHITECTURAL DRAWINGS FOR DEMOLITION WORK. 5. ALL COLUMN FOOTINGS ARE CENTERED ON COLUMNS U.N.O. ON PLAN OR IN 6. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR SLABS.
7. GENERAL CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING EXISTING DIMENSIONS, ELEVATIONS, GENERAL CONDITIONS, AND BUILDING ORIENTATION BEFORE FABRICATING STEEL. EXISTING BUILDINGS MAY HAVE MINOR MISALIGNMENT BETWEEN THEM. CONTRACTOR IS RESPONSIBLE FOR MAKING MINOR ADJUSTMENTS IN ORDER TO FIT STEEL FRAME BETWEEN EXISTING BUILDINGS. NOTIFY ARCHITECT AND ENGINEER OF ANY SIGNIFICANT DISCREPANCIES. ALL ADJUSTMENTS WILL BE APPROVED BY THE ARCHITECT AND ENGINEER. 8. SEE ARCH. DWGS., SPECIFICATIONS, AND GENERAL NOTES FOR EXTERIOR METAL STUD WALL REQUIREMENTS. METAL STUD NC REGISTERED ENGINEER IS TO DESIGN FOOTING SCHEDULE METAL STUDS AND THEIR CONNECTIONS TO STRUCTURE FOR COMPONENT AND CLADDING WIND LOADS. REINFORCING E.W. BOTTOM TYPE LENGTH WIDTH THICKNESS (U.N.O.) (7)-#5 6' - 0" (8)-#5 1' - 6" FOUNDATION PLAN NOTES

FLEMING & ASSOCIATES, PA 1004 HAY STREET FAYETTEVILLE, NC 28305 FAX 910-433-2604 NC LICENSE NO. C-2828

Architecture 654 Hay Street, Suite 4 Fayetteville, NC 28301 Phone: (410) 223-2186 E-Mall: gordone

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07/31/23

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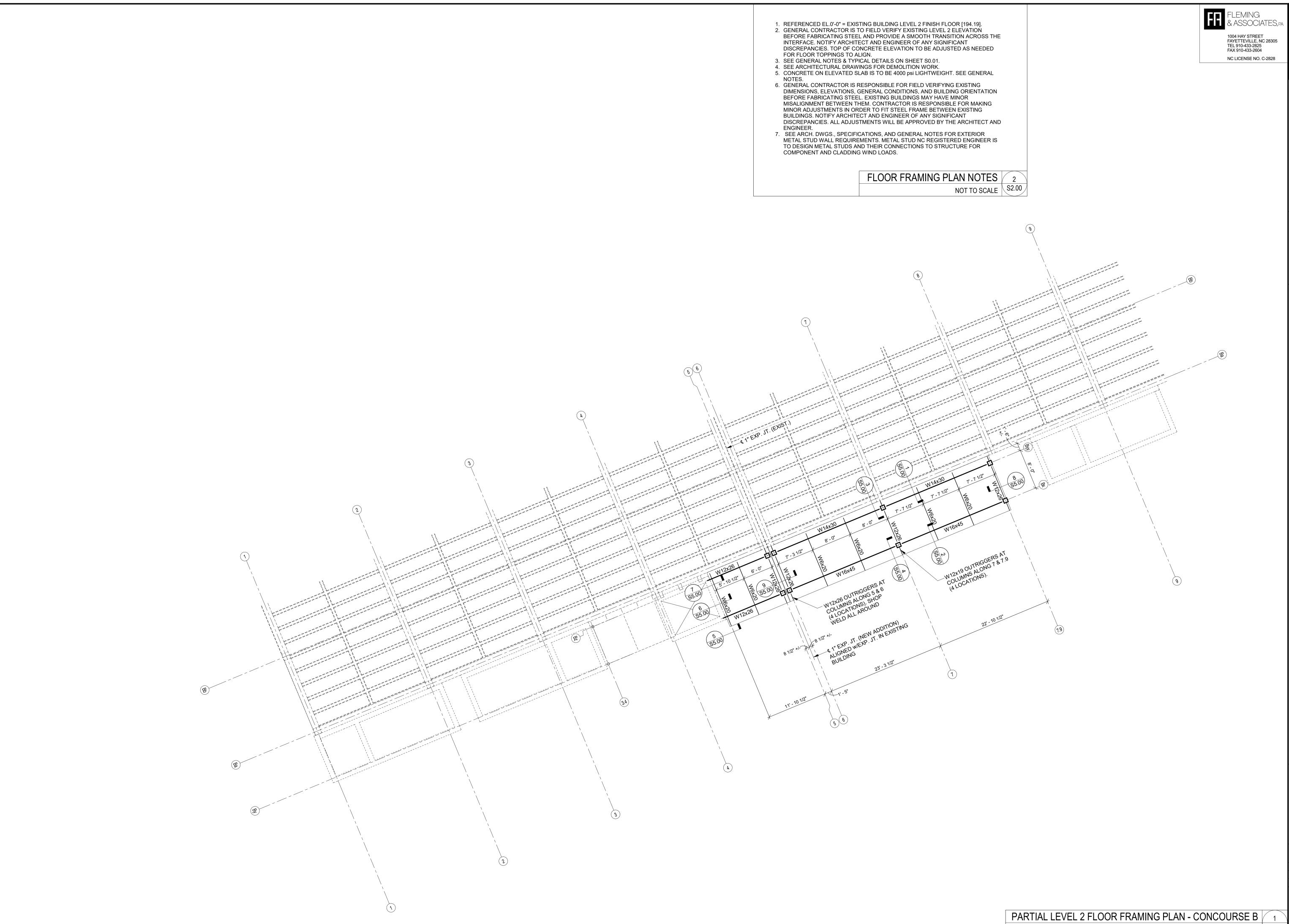
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DATE: 10/31/22

PROJECT NO: 23–58

NOTES: \_\_\_\_\_

PARTIAL FOUNDATION PLAN - CONCOURSE B





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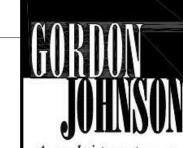
 $\sim$ PARTIAL LEVEL CONCOURSE B

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DATE: 05/17/22

PROJECT NO: 23-58



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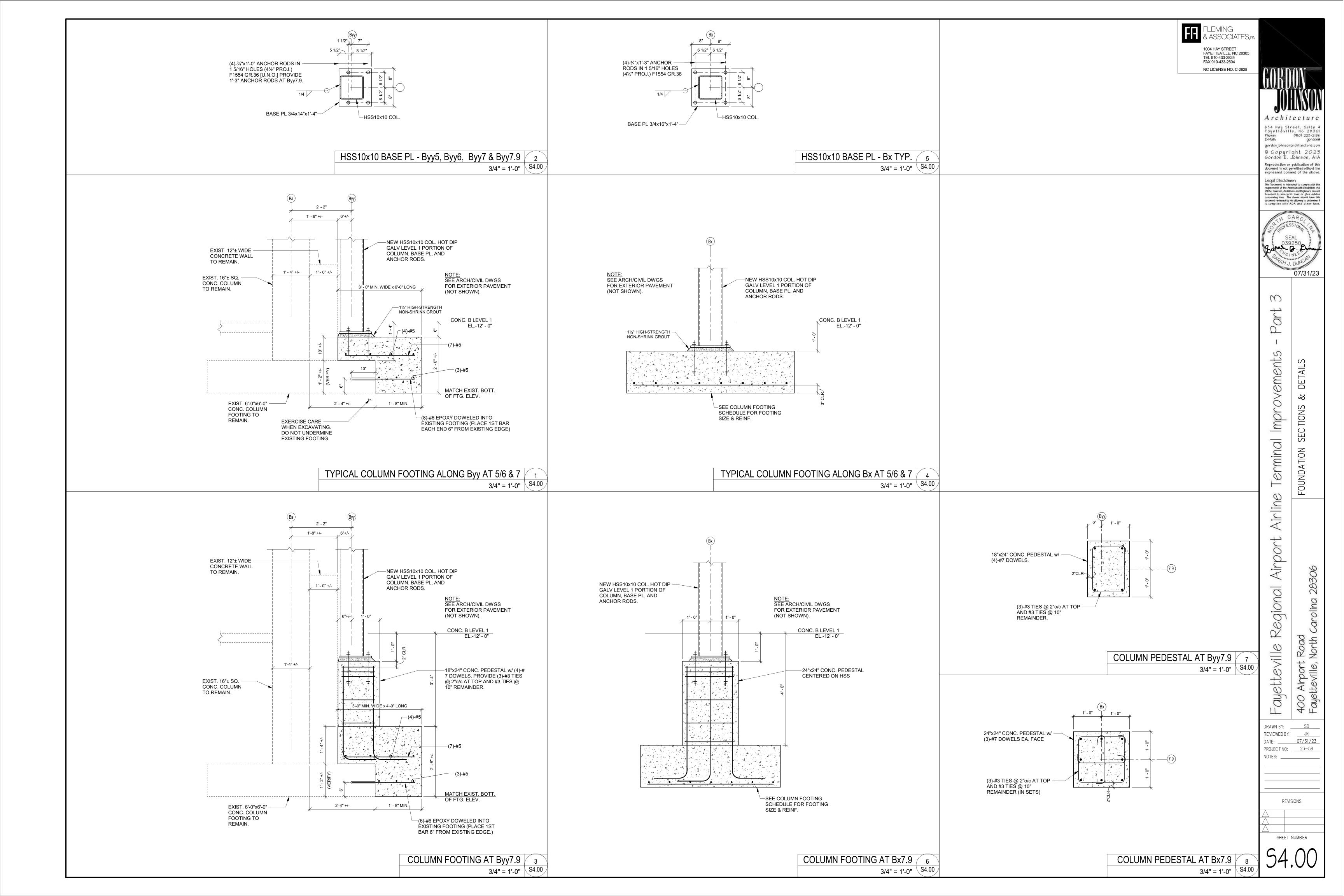
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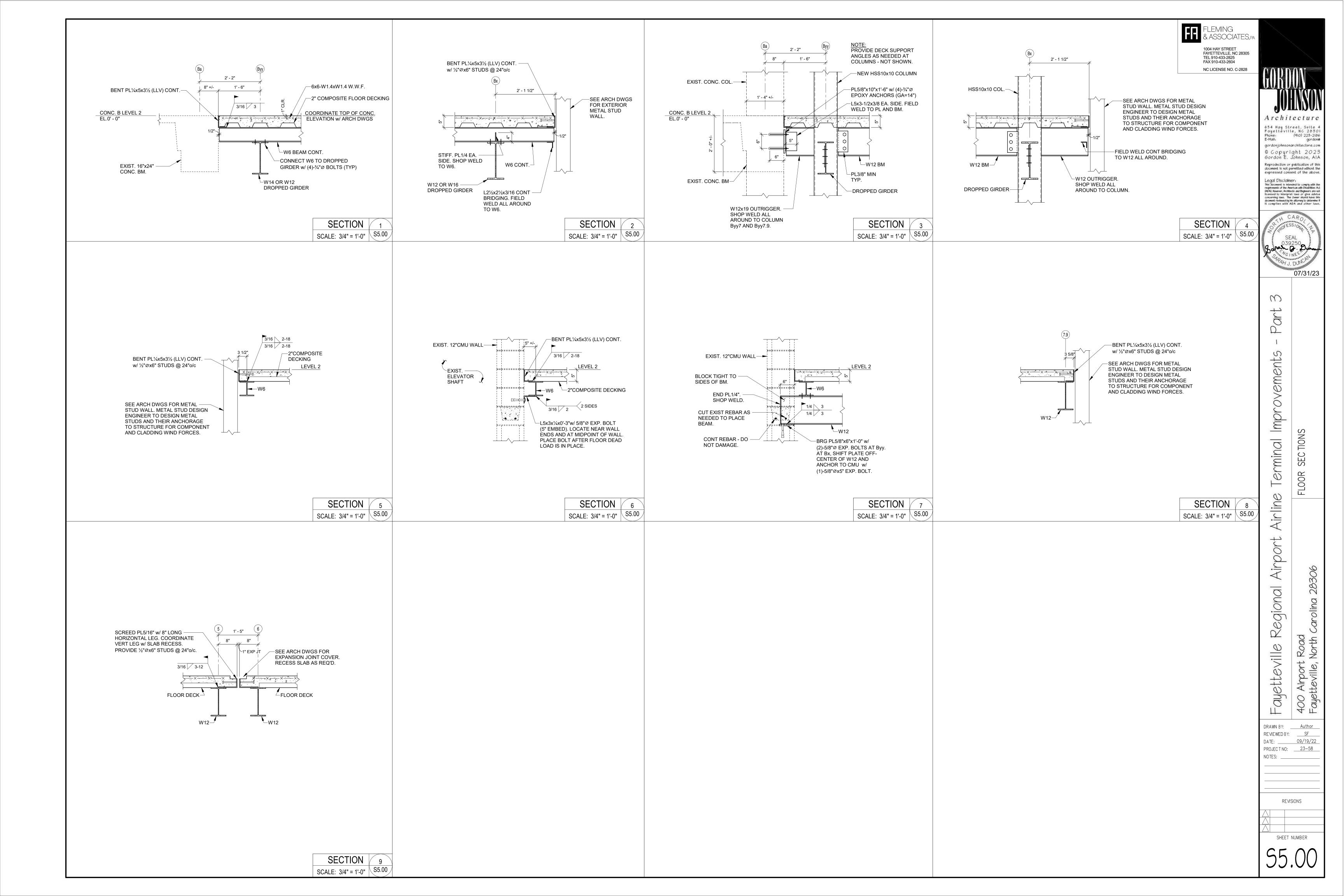
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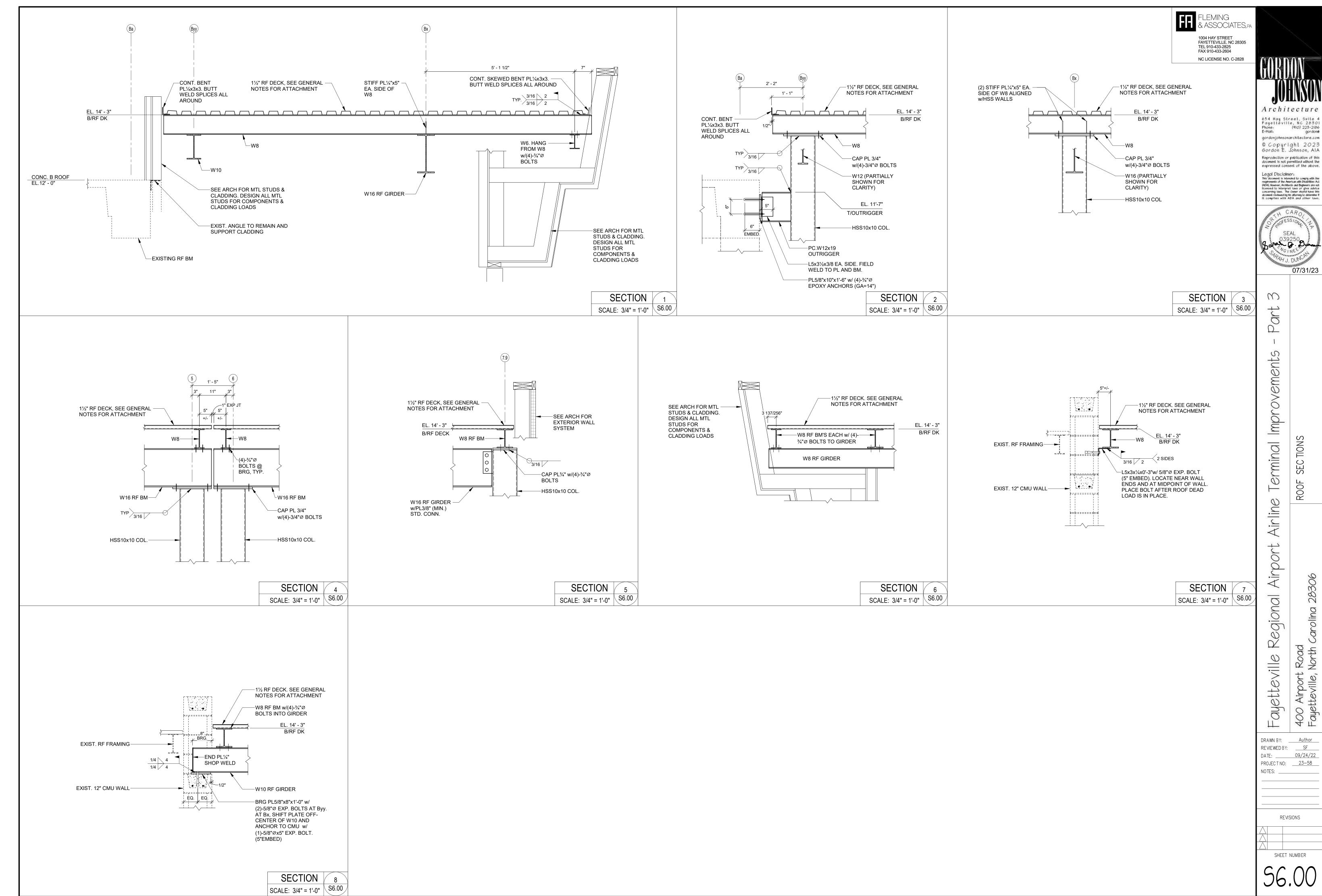
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